A. Appendix A

Wheeler Army Airfield - Field Enclosure Photographs

- 1. Construction July 2002
- 2. Installed at site May 2003
- 3. Full Structure Inspection August 18, 2003 13 Months
- 4. Full Structure Inspection March 18, 2004 20 Months

Wheeler Army Airfield – Construction – July 2002 and May 2003

Construction off-site and relocation



Figure A.1: Pre-fabrication in Pearl City – July 2002 Figure A.2: West elevation – May 2003





Figure A.3: North Elevation – May 2003



Figure A.4: South Elevation – May 2003

Crawl Space – Cripple Wall



Figure A.5: Cripple Wall – NE end



Figure A.7: Cripple Wall base – NE end



Figure A.9: Cripple wall base connection – NE end



Figure A.6: Cripple Wall – SE end



Figure A.8: Cripple Wall top – SE end



Figure A.10: Cripple wall base – SE end

Crawl Space - Posts



Figure A.11: NW Post



Figure A.13: NW Post base connection



Figure A.12: SW Post



Figure A.14: SW Post base connection

Crawl Space - Floor Joists



Figure A.15: Exposed floor joists



Figure A.17: Joist stiffener over cripple wall



Figure A.19: Double edge joist with stiffener



Figure A.16: Exposed test connections – location 3



Figure A.18: Joist connection over cripple wall



Figure A.20: Covered crawl coupons – location 4

Interior Wall Framing – East Wall – Plywood without vapor barrier



Figure A.21: East wall – North side



Figure A.23: East Wall – North Top Track



Figure A.25: East Wall – Top Track Connection



Figure A.22: East wall – South side



Figure A.24: East Wall – South Top Track



Figure A.26: East wall – Stud-track connection

Interior Wall Framing – North Wall – Plywood without vapor barrier



Figure A.27: North Wall – West side



Figure A.28: North wall – Header west connection



Figure A.29: North Wall – Top track connection

Vented Attic Framing



Figure A.30: Roof Trusses



Figure A.32: Ridge connection



Figure A.34: Ridge connection fasteners



Figure A.31: End Truss



Figure A.33: King post base connection



Figure A.35: Location 2 – Attic test connections

Test Connection Locations



Figure A.36: Location 1 – Interior wall



Figure A.39: Location 1 – Interior test connections



Figure A.37: Location 5 – Exterior exposure



Figure A.38: Exterior test connection rack

Crawl space cripple wall



Figure A.40: Cripple Wall – NE corner



Figure A.42: Cripple Wall – NE corner top



Figure A.44: Cripple Wall – NE corner bottom



Figure A.41: Cripple Wall – SE corner



Figure A.43: Cripple Wall – SE corner top



Figure A.45: Cripple Wall – SE corner bottom

Crawl space posts



Figure A.46: SW Post



Figure A.48: SW Post top



Figure A.50: SW Post base



Figure A.47: NW Post



Figure A.49: NW Post top



Figure A.51: NW Post base

Floor Joist Framing – Specimen exposure 7 Months



Figure A.52: Location 3 – Uncovered joists



Figure A.54: Location 3 – connection fasteners



Figure A.56: Location 4 – connection fasteners



Figure A.53: Location 3 – test connections



Figure A.55: Location 4 – Enclosed joists



Figure A.57: Joist connection

Interior Wall Framing – Specimen exposure 7 Months



Figure A.58: Location 1 – Interior wall



Figure A.60: Location 1 – connection fasteners



Figure A.62: 7 Month exterior connections added



Figure A.59: Location 1 – Top track connections



Figure A.61: Location 1 – zinc coupon

Vented Attic Framing – Specimen exposure 7 Months



Figure A.63: Location 2 – Attic test connections



Figure A.65: Location 2 – 7 Month conn. added



Figure A.67: 7 Month exterior connection set



Figure A.64: Location 2 – Truss top connection



Figure A.66: Location 2 – Truss bottom connection



Figure A.68: 7 Month exterior connection fasteners

B. Appendix **B**

Iroquois Point Inland - Field Enclosure Photographs

- 1. Construction July 2002
- 2. Installed at site March 2003
- 3. Full Structure Inspection August 13, 2003 13 Months
- 4. Full Structure Inspection March 18, 2004 20 Months

Iroquois Point Inland – Construction – July 2002 and March 2003

Construction off-site and relocation



Figure B.1: Pre-fabrication in Pearl City – July 2002 Figure B.2: SW elevation – March 2003





Figure B.3: East Elevation – March 2003



Figure B.4: North Elevation – March 2003

Crawl Space – Cripple Wall



Figure B.5: East Cripple Wall



Figure B.7: Cripple Wall – NE end top



Figure B.9: Cripple Wall – NE end base



Figure B.6: Cripple Wall – SE end



Figure B.8: Cripple Wall – SE end top



Figure B.10: Cripple Wall – SE end base

Crawl Space - Posts



Figure B.11: NW Post



Figure B.13: NW Post Base



Figure B.12: SW Post



Figure B.14: SW Post Base

Crawl Space – Post Details



Figure B.15: SW Post Top – Interior view



Figure B.17: SW Post Base – Interior view



Figure B.16: NW Post Top – Interior view



Figure B.18: NW Post Base – Interior view



Figure B.19: SE Corner wall tie-downs

Crawl Space – Floor Joists



Figure B.20: Joists on Cripple Wall – NE corner



Figure B.22: Joist to track fasteners



Figure B.24: Joist connections at West end



Figure B.21: Joists on cripple wall - East end



Figure B.23: Exposed Joists



Figure B.25: Joist stiffener at West connection

Interior Wall Framing – East wall – Plywood without vapor barrier



Figure B.26: East Wall – North end framing



Figure B.28: East Wall – Stud to track detail



Figure B.30: Test connections in East wall – Loc 1



Figure B.27: East Wall – Top Track detail



Figure B.29: Test connections in East wall – Loc 1

Vented Attic Framing



Figure B.31: Attic Framing



Figure B.33: King post base connection



Figure B.35: King post top connection



Figure B.32: Ridge connection fasteners



Figure B.34: King post base connection



Figure B.36: Test connections in attic – Location 2

Test Connection Locations



Figure B.37: Location 3 – Exposed crawl space



Figure B.39: Location 5 – Exterior rack



Figure B.38: Location 4 – Covered crawl space



Figure B.40: Location 5 – Exterior rack

Crawl space cripple wall



Figure B.41: Cripple Wall – NE corner



Figure B.43: Cripple Wall – NE corner top



Figure B.45: Cripple Wall – NE corner bottom



Figure B.42: Cripple Wall – SE corner



Figure B.44: Cripple Wall – SE corner top



Figure B.46: Cripple Wall – SE corner bottom

Crawl space posts



Figure B.47: SW Post



Figure B.49: SW Post top



Figure B.51: SW Post base



Figure B.48: NW Post



Figure B.50: NW Post top



Figure B.52: NW Post base

Floor Joist Framing – Specimen exposure 7 Months



Figure B.53: Location 3 – Uncovered joists



Figure B.55: Location 3 – connection fasteners



Figure B.57: Location 4 – connection fasteners



Figure B.54: Location 3 – test connections



Figure B.56: Location 4 – Enclosed joists



Figure B.58: Joist connection

Interior Wall Framing – Specimen exposure 7 Months



Figure B.59: Location 1 – Interior wall



Figure B.61: Location 1 – connection fasteners



Figure B.60: Location 1 – Top track connections



Figure B.62: 7 Month exterior connections added

Vented Attic Framing and Exterior – Specimen exposure 7 Months



Figure B.63: Location 2 – Attic truss connection



Figure B.65: Location 5 – Exterior connections



Figure B.67: Location 5 – steel coupons



Figure B.64: Location 2 – connection fasteners



Figure B.66: Location 5 – connection fasteners



Figure B.68: Location 6 – zinc coupon

C. Appendix C

Iroquois Point Coastal - Field Enclosure Photographs

- 1. Construction July 2002
- 2. Installed at site March 2003
- 3. Full Structure Inspection August 13, 2003 13 Months
- 4. Full Structure Inspection March 18, 2004 20 Months

Iroquois Coastal – Construction – July 2002 and March 2003

Construction off-site and relocation



Figure C.1: Prefabrication in Pearl City – July 2002



Figure C.2: West elevation – March 2003



Figure C.3: North elevation – March 2003



Figure C.4: West elevation – March 2003

Crawl Space Cripple Wall



Figure C.5: Cripple Wall NE end



Figure C.7: Exposed joists on cripple wall – NE



Figure C.9: Cripple wall base - NE



Figure C.6: Cripple Wall SE end



Figure C.8: Covered joists on cripple wall - SE



Figure C.10: Cripple wall base - SE

Crawl Space - Posts



Figure C.11: SW Post top



Figure C.13: SW Post base - Interior



Figure C.15: SW Post base – Exterior



Figure C.12: NW Post top



Figure C.14: NW Post base - Interior



Figure C.16: NW Post base - Exterior

Crawl Space – Floor Joists



Figure C.17: Exposed and covered floor joists



Figure C.19: Joist connection at South elevation



Figure C.21: Joist connection fasteners



Figure C.18: Joist connection over cripple wall



Figure C.20: Joist stiffener and fasteners



Figure C.22: Joist to track fasteners

Interior Wall Framing – East Wall – Plywood without vapor barrier



Figure C.23: East Wall – North End



Figure C.25: East Wall – North corner top track



Figure C.24: East Wall – South End



Figure C.26: East Wall – Stud-track connection



Figure C.27: East Wall – Test Connections

Interior Wall – South Wall – Plywood without vapor barrier



Figure C.28: South Wall – East End



Figure C.30: South Wall – Header Connection E



Figure C.29: South Wall – West End



Figure C.31: South Wall – Header Connection W



Figure C.32: South Wall – West corner fasteners

Interior Walls - North and West Walls



Figure C.33: North Wall – West top corner



Figure C.35: North Wall – Header connection W



Figure C.37: West Wall – North End



Figure C.34: North Wall – East End



Figure C.36: North Wall – Header connection E



Figure C.38: West Wall – Door stud connection
Iroquois Point Coastal – 13 Month Condition – August 13, 2003

Vented Attic Framing



Figure C.39: Attic framing with test connections



Figure C.41: Post to cross brace connection



Figure C.40: Post to ceiling joist connection



Figure C.42: Eave connection



Figure C.43: Ridge Connection

Iroquois Point Coastal – 13 Month Condition – August 13, 2003

Test Connection Locations



Figure C.44: Location 1 – Interior Wall



Figure C.46: Location 3 – Uncovered Crawl space



Figure C.48: Location 5 – Exterior Rack



Figure C.45: Location 2 – Vented Attic



Figure C.47: Location 4 – Covered crawl space



Figure C.49: Location 5 – Exterior Rack end view

Crawl space cripple wall



Figure C.50: Cripple Wall – NE corner



Figure C.52: Cripple Wall – NE corner top



Figure C.54: Cripple Wall – NE corner bottom



Figure C.51: Cripple Wall – SE corner



Figure C.53: Cripple Wall – SE corner top



Figure C.55: Cripple Wall – SE corner bottom

Crawl space posts



Figure C.56: SW Post



Figure C.58: SW Post top



Figure C.60: SW Post base



Figure C.57: NW Post



Figure C.59: NW Post top



Figure C.61: NW Post base

Floor Joist Framing – Specimen exposure 7 Months



Figure C.62: Location 3 – Uncovered joists



Figure C.64: Location 3 – connection fasteners



Figure C.66: Location 4 – connection fasteners



Figure C.63: Location 3 – test connections



Figure C.65: Location 4 – Enclosed joists



Figure C.67: Exposed joist connection

Interior Wall Framing – Specimen exposure 7 Months



Figure C.68: Location 1 – Interior wall



Figure C.70: Location 1 – connection fasteners



Figure C.69: Location 1 – Top track connections



Figure C.71: 7 Month exterior connections added

Vented Attic Framing – Specimen exposure 7 Months



Figure C.72: Location 2 – Attic



Figure C.74: Location 2 – Ridge truss connection



Figure C.76: Location 2 – Truss bottom connection



Figure C.73: Location 2 – test connections



Figure C.75: Location 2 – Ridge truss connection



Figure C.77: Truss bottom connection

Location 5 – Exterior specimen exposure 7 Months



Figure C.78: Location 5 – Test connections



Figure C.80: 7 Month test connection set



Figure C.79: Location 5 – connection fasteners



Figure C.81: 7 Month exterior exposure



Figure C.82: Location 5 – Steel and zinc coupons

D. Appendix D

MCBH Inland - Field Enclosure Photographs

- 1. Construction Nov Dec 2001
- 2. Crawl Space Inspection April 2, 2002 5 Months
- 3. Crawl Space Inspection Sept, 2002 10 Months
- 4. Full Structure Inspection March 6, 2003 16 Months
- 5. Full Structure Inspection August 11, 2003 21 Months Coupons installed
- 6. Connection Specimen Condition September 23, 2003 1.5 Month exposure
- 7. Full Structure Inspection March 11, 2004 28 Months
- 8. Connection Specimen Condition March 11, 2004 7 Month exposure

MCBH Inland – Construction – Nov-Dec 2001



Figure D.1: NE elevation during construction



Figure D.3: SW cripple wall tie-down



Figure D.5: Joist stiffeners on cripple wall



Figure D.2: SW elevation during sheathing



Figure D.4: NW cripple wall tie-down



Figure D.6: Exposed and covered joists

MCBH Inland – Construction – Nov-Dec 2002



Figure D.7: NE post and brace



Figure D.9: Roof framing



Figure D.11: NE elevation after completion



Figure D.8: Interior wall and roof framing



Figure D.10: Roof truss eave connection



Figure D.12: East elevation with weather station

MCBH Inland – 5 Month Condition – April 2, 2002

Crawl Space – Posts and Cripple Wall



Figure D.13: SW end of cripple wall



Figure D.15: NE post – top



Figure D.17: NE post - base



Figure D.14: NW end of cripple wall



Figure D.16: SE post - top



Figure D.18: SE post - base

MCBH Inland – 5 Month Condition – April 2, 2002

Floor Framing



Figure D.19: Joist ends bearing on cripple wall



Figure D.21: Exposed and covered joists - East



Figure D.20: Exposed and covered joist - West



Figure D.22: Joist and double joist over post

Crawl Space Posts and Cripple Wall



Figure D.23: SW corner cripple wall base



Figure D.25: NE corner post base



Figure D.24: NW corner cripple wall base



Figure D.26: SE corner post base

Floor Joists



Figure D.27: SW corner exposed joists



Figure D.29: NE exposed and covered joists



Figure D.31: Joist stiffener over cripple wall



Figure D.28: NW exposed and covered joists



Figure D.30: SE exposed joists



Figure D.32: Joist midspan

Interior Wall Framing – Lap siding without vapor barrier



Figure D.33: E wall – Lap siding w/o vapor barrier



Figure D.35: E wall – Stud to top track connection



Figure D.34: E wall – Studs and top track



Figure D.36: E wall – Load path tie-down

Interior Wall Framing – Plywood sheathing without vapor barrier



Figure D.37: S wall – plywood w/o vapor barrier



Figure D.39: S wall – East header connection



Figure D.38: S wall – plywood with insulation



Figure D.40: S wall – West header connection



Figure D.41: S wall – Stud to top track connection

Interior Wall Framing – Lap Siding with Vapor Barrier



Figure D.42: N wall – Lap siding with vapor barrier



Figure D.44: N wall – Header connection



Figure D.43: N wall – Lap siding with insulation



Figure D.45: N wall – Top track connections



Figure D.46: N wall – Stud to top track connection

Vented Attic - Roof Framing



Figure D.47: Typical roof truss



Figure D.49: Roof truss connection



Figure D.51: Roof truss eave connection



Figure D.48: Roof truss ridge connection



Figure D.50: Roof truss fasteners

Crawl Space



Figure D.52: SW corner cripple wall



Figure D.54: SW Cripple Wall base connection



Figure D.56: NE Post base



Figure D.53: NW corner cripple wall



Figure D.55: NW Cripple Wall base connection



Figure D.57: SE Post base

Crawl Space – Floor Joists



Figure D.58: Exposed floor joists



Figure D.60: Test coupons in location 6 - exposed



Figure D.59: Floor joist end connections



Figure D.61: Test coupons in location 7 - enclosed

Interior Wall Framing – without vapor barrier



Figure D.62: Loc. 2 – lap siding w/o vapor barrier



Figure D.64: Location 2 test connections



Figure D.63: Loc. 1 – plywood w/o vapor barrier



Figure D.65: Location 1 test connections

Interior Wall and Roof Framing



Figure D.66: Location 3 – wall with vapor barrier



Figure D.68: Location 3 – test connections



Figure D.67: Location 4 – roof framing



Figure D.69: Location 4 – test connections

Exterior Test Connections



Figure D.70: Location 7 – exterior test connections



Figure D.72: Chloride Candle and shield



Figure D.71: Location 7 – exterior test connections



Figure D.73: Chloride Candle

MCBH Inland – 24 Month Condition – November 11, 2003

Exterior Test Connections – 3 Month Exposure



Figure D.74: Location 7 – 3 Month Exposure



Figure D.76: Close-up of 3 Month exposure



Figure D.75: Location 7 – 3 Month exp fasteners



Figure D.77: Steel and zinc coupons – 3 Months

Crawl Space Cripple Wall



Figure D.78: Cripple Wall – SW corner



Figure D.80: Cripple Wall Top – SW corner



Figure D.82: Cripple Wall Base – SW corner



Figure D.79: Cripple Wall – NW corner



Figure D.81: Cripple Wall Top – Joist connection



Figure D.83: Cripple Wall Base – NW corner

Crawl Space Posts



Figure D.84: Post – NE corner



Figure D.86: Post Top – NE corner



Figure D.88: Post Base – NE corner



Figure D.85: Post – SE corner



Figure D.87: Post Top – SE corner



Figure D.89: Post Base – SE corner

Open Crawl Space Floor Joists



Figure D.90: Location 5 specimens - open crawl



Figure D.92: Joist end connection – open crawl



Figure D.94: Location 5 specimens – open crawl



Figure D.91: Joists and specimens – open crawl



Figure D.93: Joist end stiffener- open crawl



Figure D.95: 7 Month exterior exposure specimens

Covered Crawl Space Floor Joists



Figure D.96: Joists and specimens – covered crawl



Figure D.98: Connection fasteners – covered crawl



Figure D.100: 7 Month exterior exposure specimens



Figure D.97: Joists and specimens – covered crawl



Figure D.99: 7 Month exterior specimens added



Figure D.101: 7 Month exterior fasteners

Interior Wall Framing – Plywood with vinyl siding



Figure D.102: Location 1 - S wall - plywood



Figure D.104: Location 1 header connection



Figure D.106: Location 1 – test connections



Figure D.103: Location 1 – corner connection



Figure D.105: Location 1 header connection



Figure D.107: 7 Month exterior connections added

Interior Wall Framing – Lap siding without vapor barrier



Figure D.108: Location 2 - E wall – Lap siding



Figure D.109: Location 2 – test specimens



Figure D.110: Location 2 – top plate connection



Figure D.112: Location 2 – test connections



Figure D.111: Location 2 – top plate connection



Figure D.113: 7 Month exterior connections added

Interior Wall Framing – Lap siding with vapor barrier



Figure D.114: Location 3 - N wall – vapor barrier



Figure D.116: Location 3 – Header connection



Figure D.118: Location 3 – ext connections added



Figure D.115: Location 3 – corner connection



Figure D.117: Location 3 – Header connection



Figure D.119: 7 Month exterior connections added

Vented Attic Framing – Location 4



Figure D.120: Location 4 – test connections



Figure D.122: Location 4 – connection fasteners



Figure D.124: Location 4 – ext connections added



Figure D.121: Location 4 – top truss connection



Figure D.123: Location 4 – bottom truss connection



Figure D.125: 7 Month exterior connections added

Exterior test connections – Location 7



Figure D.126: Location 7 – test connections



Figure D.128: Location 7 – fasteners heads



Figure D.130: Location 7 – test set fasteners



Figure D.127: Location 7 – fasteners threads



Figure D.129: Location 7 – test connection set



Figure D.131: 7 Month exterior fasteners

E. Appendix E

MCBH Coastal - Field Enclosure Photographs

- 1. Construction Nov Dec 2001
- 2. Crawl Space Inspection April 2, 2002 5 Months
- 3. Crawl Space Inspection Sept, 2002 10 Months
- 4. Full Structure Inspection March 6, 2003 16 Months
- 5. Full Structure Inspection August 11, 2003 21 Months Coupons installed
- 6. Connection Specimen Condition September 23, 2003 1.5 Month exposure
- 7. Full Structure Inspection March 11, 2004 28 Months
- 8. Connection Specimen Condition March 11, 2004 7 Month exposure

University of Hawaii Steel Framing Alliance

MCBH Coastal – Construction – Nov-Dec 2001



Figure E.1: Concrete Footings



Figure E.3: Framing on site



Figure E.5: Completed Enclosure - NE Elevation



Figure E.2: Foundation location



Figure E.4: Foundation Tiedowns



Figure E.6: Completed Enclosure – SE Elevation
MCBH Coastal – 5 Month Condition – April 2, 2002

Crawl Space – Posts and Cripple Wall



Figure E.7: Top of NW post



Figure E.9: Bottom of NW post



Figure E.11: Bottom of Cripple Wall - SE



Figure E.8: Bottom of NE post



Figure E.10: Bottom of Cripple Wall - SW



Figure E.12: Cripple Wall Tiedown

MCBH Coastal – 5 Month Condition – April 2, 2002

Crawl Space Joists



Figure E.13: Exposed Floor Joist – S Wall



Figure E.15: Double Joist at SW corner



Figure E.17: Exposed Joist – N end - E of center



Figure E.14: Exposed Floor Joist Stiffener



Figure E.16: Exposed Joist – N end - Center



Figure E.18: Exposed Joist - Midspan

MCBH Coastal – 10 Month Condition – September 2002

Crawl Space – Exposed Joists



Figure E.19: Exposed Joists and Cripple Wall



Figure E.20: Exposed Joists and Wall - SW



Figure E.21: Exposed Joists Midspan – West

Crawl Space Posts and Cripple Wall



Figure E.22: Post Base – NW corner



Figure E.24: Cripple Wall Base – SE corner



Figure E.23: Post Base – NE corner



Figure E.25: Cripple Wall Base – SW corner

Crawl Space Floor Joists



Figure E.26: Exposed Floor Joists - SE



Figure E.28: Exposed Floor Joists - SW



Figure E.30: Exposed Floor Joists - NE



Figure E.27: Exposed Floor Joists - South



Figure E.29: Exposed Floor Joists - NW



Figure E.31: Exposed Floor Joists – Midspan - E

Floor Joist Details



Figure E.32: Joist connection at cripple wall



Figure E.34: Joist stiffener at South end



Figure E.36: Exposed joist detail at midspan



Figure E.33: Joist stiffener at cripple wall



Figure E.35: Joist connection at South end



Figure E.37: Exposed joist soffit at midspan

Enclosed Floor Joists



Figure E.38: Enclosed floor joist punch-out



Figure E.39: Enclosed floor joist detail

Interior Wall Framing – Lap siding without vapor barrier



Figure E.40: W wall – Lap siding w/o vapor barrier



Figure E.42: W wall header connection



Figure E.44: W Wall top track connection



Figure E.41: W wall – Lap siding with insulation



Figure E.43: W wall top track



Figure E.45: W wall lap siding w/o vapor barrier

Interior Wall Framing – Plywood sheathing without vapor barrier



Figure E.46: E wall – Plywood w/o vapor barrier



Figure E.48: E wall – Top track



Figure E.47: E wall – Header connection



Figure E.49: E wall – Sheathing & stud fasteners

Interior Wall Framing - with vapor barrier



Figure E.50: N wall – vapor barrier & insulation



Figure E.53: N wall – stud and siding fasteners



Figure E.51: N wall – top track



Figure E.52: N wall – stud to top track connection

Vented Attic Framing



Figure E.54: Vented eave overhang



Figure E.56: Ridge connection



Figure E.58: Eave connection fasteners



Figure E.55: Roof truss



Figure E.57: Ridge connection fasteners



Figure E.59: Ceiling to king post fasteners

Crawl Space – Posts and Cripple Wall



Figure E.60: Post Base – NW corner



Figure E.62: Cripple Wall Base – SE corner



Figure E.64: Close-up SE corner



Figure E.61: Post Base – NE corner



Figure E.63: Cripple Wall Base – SW corner



Figure E.65: Close-up cripple wall tie-down

Crawl Space Floor Joists



Figure E.66: Exposed Joists – looking North



Figure E.68: Exposed Joists – SW corner



Figure E.70: Exposed Joists – Close-up of fastener



Figure E.67: Exposed Joists – looking South



Figure E.69: Exposed Joists - Midspan



Figure E.71: Exposed Joists – Midspan corrosion

Floor Joist Details



Figure E.72: Joists bearing on cripple wall



Figure E.74: Joist stiffener at North end



Figure E.76: Double joist at NW post



Figure E.73: Joist stiffener over cripple wall stud



Figure E.75: Double joist at cripple wall post



Figure E.77: Cripple wall below covered joists

Covered Floor Joists



Figure E.78: Exposed and covered floor joists



Figure E.80: Covered Floor Joist – North end



Figure E.82: Corrosion around opening in joist



Figure E.79: Covered joist and fastener



Figure E.81: Covered Floor Joist – South end



Figure E.83: Covered joist on exposed top track

Interior Wall Framing – Lap siding without vapor barrier



Figure E.84: W wall – Lap siding w/o vapor barrier



Figure E.85: W Wall – Lap siding with insulation



Figure E.86: W wall header connection - S



Figure E.88: W wall – S corner stud fasteners



Figure E.87: W wall header connection - N



Figure E.89: W wall – top track detail

Interior Wall Framing – Plywood sheathing without vapor barrier



Figure E.90: E wall – Plywood w/o vapor barrier



Figure E.92: E wall – Header and stud fasteners



Figure E.94: E wall – N corner stud fasteners



Figure E.91: E wall – Plywood with insulation



Figure E.93: E wall – Header connection



Figure E.95: E wall – top track fasteners

Interior Wall Framing – with vapor barrier



Figure E.96: N wall – vapor barrier & insulation



Figure E.98: N wall – top track connection



Figure E.97: N wall – with vapor barrier



Figure E.99: N wall – Truss tie-down connection



Figure E.100: N wall – Header and stud connection

Vented Attic Framing



Figure E.101: Truss Connection



Figure E.103: Ridge connection



Figure E.105: Eave connection fastener heads



Figure E.102: Truss Connection – fastener heads



Figure E.104: Ridge connection fasteners



Figure E.106: Eave connection fastener threads

MCBH Coastal – 21 Month Connection Placement – August 11, 2003

Placement of Test Connection Specimens



Figure E.107: Location 1 – East wall



Figure E.109: Location 3 – West Wall



Figure E.108: Location 2 – North wall



Figure E.110: Location 4 – Attic

MCBH Coastal – 21 Month Connection Placement – August 11, 2003

Placement of Test Connection Specimens



Figure E.111: Location 5 – Exposed crawl space



Figure E.113: Location 7 – External specimens



Figure E.115: External specimen rack



Figure E.112: Location 6 – Covered crawl space



Figure E.114: External connection specimen rack

MCBH Coastal – Connection Specimen Condition – September 23, 2003

42 Day exposure



Figure E.116: Location 1 – East Wall



Figure E.118: Location 3 – West Wall



Figure E.120: Location 5 – Exposed Crawl Space



Figure E.117: Location 2 – North Wall



Figure E.119: Location 4 - Attic



Figure E.121: Location 6 – Covered Crawl Space

MCBH Coastal – Connection Specimen Condition – September 23, 2003

42 Day exposure



Figure E.122: Location 7 – External screw threads



Figure E.123: Location 7 – External screw heads



Figure E.124: Location 7 – External specimens

Crawl Space Cripple Wall



Figure E.125: Cripple Wall - SE corner



Figure E.127: Cripple Wall Top – SE corner



Figure E.129: Cripple Wall Base – SE corner



Figure E.126: Cripple Wall – SW corner



Figure E.128: Cripple Wall Top – SW corner



Figure E.130: Cripple Wall Base – SW corner

Crawl Space Posts



Figure E.131: Post – NW corner



Figure E.133: Post Top – NW corner



Figure E.135: Post Base – NW corner



Figure E.132: Post – NE corner



Figure E.134: Post Base – NE corner

Crawl Space Floor Joists



Figure E.136: Exposed Floor Joists - SW



Figure E.138: Joist Connection at cripple wall



Figure E.140: Joist stiffener at South end



Figure E.137: Exposed Floor Joists – Close-up



Figure E.139: Joist stiffener at cripple wall



Figure E.141: Joist connection at South end

Interior Wall Framing – Lap siding without vapor barrier



Figure E.142: W wall – Lap siding w/o barrier



Figure E.144: W wall header connection - left



Figure E.146: W wall corner connection



Figure E.143: W wall- Lap siding with insulation



Figure E.145: W wall header connection - right



Figure E.147: W wall top track connection

Interior Wall Framing – Plywood and lap siding with vapor barrier



Figure E.148: E wall – Plywood w/o barrier



Figure E.150: E wall – Test connections



Figure E.152: N wall – Test connections



Figure E.149: E wall - Header connection



Figure E.151: N wall – vapor barrier & insulation



Figure E.153: N wall - top track connections

Vented Attic Framing



Figure E.154: Attic test connections



Figure E.156: Roof truss connection



Figure E.158: Roof truss connection



Figure E.155: Ridge connection



Figure E.157: Ceiling to king post fasteners



Figure E.159: Fastener heads at truss connection

MCBH Coastal – 7 Month Connection Condition – March 11, 2004

Test connections



Figure E.160: Open crawl space



Figure E.162: Open crawl space – steel coupons



Figure E.164: Covered crawl space



Figure E.161: Open crawl space – close-up



Figure E.163: Open crawl space – zinc coupons



Figure E.165: Covered crawl space – connections

MCBH Coastal – 7 Month Connection Condition – March 11, 2004

Exterior test connections

Figure E.166: Exterior connections – 7 months



Figure E.167: Exterior connections - Threads



Figure E.168: Exterior connections - Heads

Corrosion of Galvanized Fasteners Final Report University of Hawaii Steel Framing Alliance

F. Appendix F

Literature Review Report

December 27, 2000

LITERATURE REVIEW REPORT

December 27, 2000

SUBMITTED BY

NORTH AMERICAN STEEL FRAMING ALLIANCE AND UNIVERSITY OF HAWAII

HUD CONTRACT No.:	H-21248CA
PROJECT TITLE:	"Corrosion of Galvanized Fasteners used in Cold-Formed Steel Framing"
CONTRACTOR:	North American Steel Framing Alliance 1726 M Street, NW, Suite 601 Washington, DC 20036
Principal Investigator: Co/Investigators:	Tim Waite Don Moody, Kevin Bielat
SUB-CONTRACTOR:	University of Hawaii Dept. of Civil Engineering 2540 Dole Street, Holmes Hall 383 Honolulu, HI 96822
Principal Investigator	Dr. Ian N. Robertson

PROJECT PERIOD:

From 9/26/2000 to 11/25/2003

LITERATURE REVIEW REPORT:

December 27, 2000

CORROSION OF GALVANIZED FASTENERS USED IN COLD-FORMED STEEL FRAMING

LITERATURE REVIEW REPORT

December 27, 2000

Section I - Introduction

This research program will investigate the potential for corrosion of galvanized fasteners used in cold-formed steel framing (CFSF) by exposing test samples to a variety of environmental conditions frequently found in Hawaii. The results of this research will aid in the evaluation of galvanized CFSF fasteners in various exposure conditions.

The project was initiated on September 26, 2000 by an award from the Department of Housing and Urban Development (HUD) to the North American Steel Framing Alliance (NASFA). The project includes a research effort to study the effects of corrosion of galvanized fasteners on CFSF connection behavior, followed by a final report and development of a Practice Guide for use by industry. NASFA has subcontracted the research component of this study to the Civil Engineering Department at the University of Hawaii (UH), a non-profit State of Hawaii educational institution. The principal investigator at UH is Dr. Ian N. Robertson, Associate Professor of Structural Engineering.

The project has a two-year duration with various scheduled deliverables, including a literature review report and quarterly progress reports. This literature review report presents research literature relevant to this study. After a general introduction to galvanic corrosion and cathodic protection provided by zinc coatings on steel, the report focuses on research performed on connections in CFSF structures.

Section II - Galvanic Corrosion

Steel stores energy when it is changed from its natural state into the metallic form used in industry. This energy later returns in the form of corrosion. Corrosion is therefore the natural transformation of manmade metals to their original state (National Association of Corrosion Engineering, 1984).

Galvanizing of steel is the most economical and effective way to protect steel. This galvanizing is an adherent coating of zinc and zinc-iron alloys on the surface of steel that provides long term protection from corrosion. Galvanizing of steel is accomplished by immersing the member in molten zinc. This immersion forms a metallurgical bond between the steel and zinc coating. The standard galvanized coating is composed of pure zinc and a very small amount of aluminum. Zinc-rich paints that coat the steel framing must satisfy three important conditions for the galvanic process to occur. The zinc particles must be in electrical contact with each other. The zinc particles must also be in electrical contact with the steel. Finally, a continuous electrolyte must exist between the zinc particles and steel (AISI, 1996; Zhang, 1997).

The zinc coating applied to the steel provides a physical barrier as well as a cathodic protection against corrosion. In most environments, zinc corrodes less than steel. The rate of corrosion of zinc in atmospheric conditions is less than one tenth of that for steel (Zhang, 2000b). In fact, atmospheric conditions particularly detrimental to steel corrosion are those in which zinc coatings have been shown to be most effective (Zhang, 2000b). These conditions include marine and industrial atmospheric exposure.

As explained by Zhang (2000a), "galvanized (ie. zinc coated) steel is a typical example of metallic coating that provides a barrier layer to protect the steel and also sacrificially protects the locations where discontinuities occur in the coating." He adds that "... galvanic corrosion resulted in a reduction of the corrosion of steel by 3 times in rural, 40 times in industrial, and 300 times in seacoast industrial atmospheres." This galvanic effect is caused because zinc acts as the sacrificial anode protecting steel, the cathode. The zinc coating on the steel members carries out the cathodic protection because it becomes the sacrificial material. This sacrificial corrosion of the zinc coating is generated because zinc is more electronegative than steel.

This galvanic protection is effective over a short distance from a discontinuous edge of the zinc coating. This Galvanic Protection Distance (PD) varies depending on the environmental conditions. A PD of up to 5 mm was observed under full immersion in deionized water. In atmospheric conditions the PD is considerable smaller, and depends on the presence of an electrolyte to facilitate anodic sacrificial corrosion thus protecting neighboring cathodic material (Zhang, 2000c). In cold-formed steel framing there are often discontinuities in the zinc coating. This is particularly evident at cut ends, drilled holes and connections. It is important that the protection distance is adequate to prevent rapid corrosion of exposed steel surfaces.

Although a galvanized coating is a great protection and cathodic barrier, the zinc coating, that is part of the galvanized coating, corrodes slowly over time. The galvanic corrosion rate of zinc and extent of galvanic protection of steel is based on dimensions and environmental factors. The type of wetness and concentration of atmospheric pollutants affect the rate of corrosion of zinc. This corrosion directly affects the durability of the galvanized steel, because it leaves the steel underneath
vulnerable to corrosion attack (AISI, 1996; Zhang, 1997). According to Zhang (2000d), "the high corrosion resistance of zinc is largely due to the formation of a stable, tenacious and compact corrosion product layer during the corrosion processes in atmospheric environments. The protective corrosion product layer is formed under the effect of cyclic weathering over a period of time." He points out that for accurate simulation of field conditions during accelerated laboratory testing, it is important that wetting and drying cycles be followed to allow the corrosion product layer to form. "The corrosion rates will be high under the conditions where tenacious and compact corrosion (products) cannot form" (Zhang, 2000d) such as during a continuous salt spray test.

Prediction of the life of a zinc coated steel member depends on various properties of the atmospheric environment. According to Zhang and Hwang (2000), "the corrosion rate of zinc in atmospheric environments may vary from as low as about 0.1 μ m/year in indoor environments to as high as more than 10 μ m/year in some industrial or marine environments This means that a G60 galvanized steel, about 13 μ m coating each side, would have a corrosion life of more than 100 years in the least corrosive environment but only for about one year in an extremely corrosive environment."

Section III - Cold-Formed Steel Construction

Cold-formed light gauge steel frames have the same corrosion issues as other types of construction materials. Moisture and pollutants can reduce the life of the coated steel as well as the fasteners. The configuration of a steel track acts similar to a channel, which allows rainwater to collect prior to construction. After construction the steel track can be located in walls that collect moisture from the ambient relative humidity. This collection of the water can produce corrosion on the track. Corrosion of the steel in the frame of a building is undesirable deterioration and has adverse effects on the structural integrity of the framing (LGSEA, 1999).

Although most galvanized steel framing in residential homes is enclosed in walls, corrosion can still occur. In time, a moisture film can form on the galvanized steel on account of the relative humidity in the air. The degree of corrosion depends on the severity of the humidity in the atmosphere. When the humidity is above 70 percent, moisture will precipitate on the steel surface (Zhang, 1997).

Light gauge steel connections are primarily constructed with externally threaded fasteners. Tapping screws are capable of drilling holes into the metal with their own threads. There are two types of tapping screw that are used in the construction of residential framing, namely self-drilling screws and self-piercing screws. When choosing the screws for the structure, two basic questions must be answered. The first question is what two materials are going to be joined. The two possible answers are steel to steel and steel to a rigid material. The answer to this question allows the engineer to choose a head style from the many different types available. The steel to steel connections requires a head with a bearing surface on the top of the material being connected. The hex washer head and the pancake head are the most frequently used in steel to steel construction.

The second question is what is the total thickness of the material being connected. The total thickness of the material that the screw is being fastened into is needed to determine the point type of the screw. The two most commonly used point types in construction are self-drilling and self-piercing (LGSEA, 1997).

The durability of the fasteners is hard to determine and design manuals do not offer substantial guidance. The life of the zinc coating on fasteners depends on the coating thickness and the environment to which it is exposed. Atmospheric and accelerated tests are good guides for the rating and coating of fasteners used in construction. Both of the atmospheric and accelerated tests should be performed, because some coating systems passed or performed well in accelerated tests did very poor in real world applications, and vise versa. (LGSEA, 1999; Roberts 1999).

Accelerated test methods for fasteners include the salt spray test. This is described in ASTM standard B-117. This practice provides a controlled corrosive environment, which produces relative corrosion resistance information for coated metals. The salt spray test apparatus consists of a fog chamber, a salt solution reservoir, a supply of compressed air, stabilizing nozzles, specimen supports, and necessary means of control. Continuous exposure to salt spray without drying periods to allow corrosion products to form a protective layer may not accurately represent atmospheric corrosion conditions (Zhang, 2000d).

The Mebon Prohesion test is similar to the Salt Spray (fog) test but it includes a drying cycle. This wetting and drying simulates long term natural exposure. The third test is the Kesternich test that is used to test heavy industrial exposure. The test involves hanging the samples in an environment of sulfur dioxide and warm water alternated with ambient conditions. The light gauge steel buildings however are exposed to a combination of the salt spray, Kesternich, and humidity all at the same time (LGSEA, 1999; ASTM, 1998).

Section IV - Atmospheric Exposure

Atmospheric corrosion is the most predominant type of corrosion for zinc coated steel. This type of corrosion can be tested in both exposure tests and simulated laboratory tests. The type of wetting, which includes duration and form of

wetness, is important in determining corrosion. An outdoor type of wetting occurs when the coated steel is exposed to rain before erection. A metal that goes through cycles of wetting and drying will allow pollutants and corrosion products to dry on the exposed metal. In seacoast areas, sea salts are deposited on the zinc-coated metal by wind and raindrops (Zhang, 1997).

Exposure tests can be accomplished by building shelters at different distances from the ocean and observing these structures for a minimum of two years. The exposure sites should vary in topography, winds, and breaking surf conditions. A FEMA study of galvanized metal connectors used in timber framed housing construction identified three corrosion locations (FEMA 1996). Oceanfront buildings (less than 100 meters from the shore) have ocean salts and humidity that accelerate the corrosion rate of the cold-formed steel framing. Buildings that are 100 to 1000 meters from the shoreline are also prone to corrosion, but at a reduced rate. Buildings that are farther inland are not prone to ocean spray and therefore experience limited corrosion (FEMA 1996; Roberts 1999).

This report also identifies five types of corrosion exposure for metal connectors in a building. In order to study all possible exposure conditions, test shelters should be designed with each of these types of exposure.

The first type of corrosion exposure is the boldly exposed exterior portion of the shelter. This exposure consists of exterior connectors that are fully exposed to the elements. The side of the shelter that faces the ocean is likely to corrode faster than the sections facing away from the ocean. Although the connectors and exterior walls are coated with large amounts of salt spray, the exterior sections are also exposed to sunlight and rain. This exposure reduces the rate of corrosion because the walls and connectors are fully dried between wettings. Drying slows the rate of corrosion (FEMA 1996).

The second type of corrosion exposure is a partially sheltered exterior exposure. This exposure consists of crawl spaces, underneath exposed roof eaves, or exterior storage areas. The corrosion rate of this exposure is worse than that of the exterior exposure. Although the partially sheltered exterior exposures receive almost as much salt spray as bold exposures, they do not receive the cleansing rain. An additional factor causing the higher rate of corrosion is that this exposure condition has a higher duration of surface wetness. Certain levels of surface wetness can cause accelerated corrosion (FEMA 1996).

A vented enclosed exposure is classified as attic spaces. The corrosion in this area varies with the location of the connector. Connectors near the exterior vents behave similar to the partially sheltered exterior exposure. For connectors that are away from the vents, or covered by insulation; the corrosion rate is lower. Unvented enclosed exposure similar to the wall framing and closed floor system has limited airflow and incoming salt spray. The corrosion rate for this unvented exposure is expected to be lower than the three previous exposure conditions.

The last enclosure condition is the interior living space exposure. In many locations, this area is sealed from most salt spray. The heated and cooling of this space reduces the interior humidity needed for corrosion. This exposure should have the lowest corrosion rate of all the exposures (FEMA 1996). However, in Hawaii and other tropical locations, through flow of air is used to moderate internal temperature rather than air-conditioning and heating. This will permit ingress of moisture and air-borne salt, and may accelerate corrosion of exposed connections.

Section V - Fastener Corrosion

Australian Standard 3566- Screws - Self Drilling – For the building and Construction Industries was recently adopted in Australia as the durability standard for building fasteners. This standard reviews the durability of fasteners and discourages the use of low-cost poor quality fasteners in buildings. The standard was developed and adopted because sheet metal protection systems had improved to the point where fastener life was often the determining factor in the longevity of a steel clad building.

This Australian standard evaluates the performance of fasteners by conducting accelerated weathering test. The various tests that fasteners must endure are fifteen cycles of Kesternick testing, one thousand hours of salt spray testing, two thousand hours of QUV and 1000 hours of humidity cabinet testing. Hot-dip Galvanized screws are exempt from the accelerated testing if they have an average 40 micron zinc thickness with a minimum zinc thickness of 35 microns. The standard also specifies that 40 micron zinc coated fasteners should be used for tropical high humidity environments. The standard classifies fasteners into three categories. Class 1 screws are used for internal applications only. Class two screws are used for general use, but not externally. Class three screws are for external use, but are not approved for corrosive external environments.

The Newcastle Branch of the Australian Corrosion Association conducted a major seminar on Preventing Corrosion of Building Fasteners in Australia to discuss the standards and important points in reducing the effects of atmospheric corrosion. Important presentations at this seminar are summarized in the August 1994 edition of Corrosion Management (1994).

Udo Buecher of BHP Sheet & Coil presented a paper on fasteners for steel cladding (Corrosion Management, 1994). Before the new standards, roofs were

fixed with fasteners with yellow chromate 8 micron zinc-plated self-drilling screws, but they presented poor corrosion resistance. The yellow chromate zinc-plated fasteners presented a poor performance in one to five years. In certain environments, corrosion of cladding can occur near the fasteners. Low quality plated products with reduced zinc-coating thickness represent a weak link in connections; therefore, adequate corrosion resistance and compatibility should be used to choose the correct fasteners. Inadequate corrosion resistance of fasteners were found to be problematic when the screws were not compatible with the cladding material, and when the environment of the installation was highly corrosive (Corrosion Management, 1994).

A recent Japanese study tested the corrosion resistance of zinc-coated sheets and connections in steel-framed houses (Honda & Nomura, 1999). The researchers subjected various types of zinc-coated steel sheets to outdoor and indoor environments. They report that hot-dip galvanized steel sheet corrosion rate was low, as anticipated. The corrosion rate of the indoor sheets was considerably slower than that of the outdoor sheets. These results confirmed that a steel-framed house has a mild corrosion environment (Honda & Nomura, 1999).

The durability of joining methods applicable to steel-framed houses was also explored. Tests on the durability of self-tapping screw connections were conducted. Self-taping screws with a known coating thickness of 20 microns were used to join galvanized steel sheets. Each specimen consisted of two 150x 60mm steel sheets which were connected in a lap splice using two self-tapping screws. The maximum shear strength (from tension test on the connected plates) was measured before and after accelerated cyclic corrosion tests. The accelerated cyclic corrosion tests were conducted by exposing the specimens to daily cycles of salt spray, drying, wetting and freezing. The specimens were subjected to the cyclic corrosion testing for eight weeks. Every two weeks during this eight-week test the shear strength of the connected sheets was evaluated. The hot-dip galvanized steel sheets developed red rust after two weeks; and lost all zinc coating after four weeks. The self-tapping screws that connected the sheets showed signs of red rust after two weeks of testing. After four weeks of testing, the screws red rust had formed over the entire screw. However, the maximum shear strength of the tested connections only declined slightly after eight weeks of cyclic corrosion testing (Honda & Nomura, 1999).

Daudet (2000) reports on tests of cold-formed steel splice connections using self-drilling screws. The single-lap shear connections produced a failure normally known as tilting/bearing. The failure results from bearing failure of the steel plate adjacent to the screw and from tilting and eventual pull-out of the screw threads. Daudet reports a group effect which reduces the effectiveness of multi-screw connections from the strength anticipated for an equal number of individual screws. He found that connections with two screws oriented transverse to the loading

direction had similar strength to single screw connections, while two screws oriented longitudinally had a 20 percent reduction in strength.

As similar study by LaBoube and Sokol (2000) also identified a significant group effect. However, varying the pattern in which the screws were arranged did not have a substantial effect on the connection strength. Failure of the steel plates is possible in connections with a large number of screws with minimum spacing and edge distances.

Section VI - References and Additional Literature

AISI, 1996. "Durability of Cold-Formed Steel Framing Members", publication RG-9605, American Iron and Steel Institute, Washington, DC, October 1996, pp. 12.

ASTM B117, 1998. "Standard Practice for Operating Salt Spray (Fog) Apparatus" American Society for Testing and Materials.

Corrosion Management, 1994, "Preventing Corrosion of Building Fasteners" synopsis of presentations made at Australasian Corrosion Association seminar.

CSIRO, 2000. "New CSIRO Accelerated Fastener Test Enhances Steel Roofing Performance", Corrosion Management, July 2000, pp. 29-32. (Abbreviated version of paper by G. A. King and U. Buecher presented at Autralasian Corrosion Conference, Manly NSW, Nov. 1999)

Daubet, Randy, L., 2000. "Behavior and Design of Self-Drilling Screw Connections", LGSEA Research Note No. 2-00, Light Gauge Steel Engineers Association, Nashville TN, pp. 4.

FEMA, 1996. "Corrosion Protection for Metal Connectors in Coastal Areas." Technical Bulletin 8-96, FIA-TB-8, Federal Emergency Management Agency / Mitigation Directorate, Washington, DC, pp. 12.

Honda, K., and Nomura, H., 1999. "Corrosion Environment and Durability of Steel-Framed Houses", Nippon Steel Technical Report No. 79, Japan, pp. 28-34.

LaBoube, R. A., and Sokol, Marc, A., 2000. "Behavior of Screw Connections", LGSEA Research Note No. 1-00, Light Gauge Steel Engineers Association, Nashville TN, pp. 4.

LGSEA, 1997. "Screw Fastener Selection for Light Gauge Steel Frame Construction", Technical Note (565c), February 1996, Light Gauge Steel Engineers Association, Nashville, TN, pp. 4. LGSEA, 1999. "Fastener Corrosion", Technical Note (560-b5) April 1999, Light Gauge Steel Engineers Association, Nashville, TN, pp. 4.

National Association of Corrosion Engineers, 1984. "Corrosion Basics, an Introduction." National Association of Corrosion Engineers, Chicago, Illinois.

Roberts, David. 1999. "A New Standard of Durability for Self-Drilling Building Fasteners", Corrosion Management.

Sakumoto, Y., Nomura, H., Matsumoto, M., Ninomiya, A., Miyao, T., and Sakamoto, Y., 2000 "Durability of Galvanized Light-Gauge Steel Shapes" draft copy of paper submitted for publication in ASCE Structural Engineering Journal.

Vought, K., 1995, "Galvanizing - Light Gauge Steel Corrosion Protection", Newsletter of the Light Gauge Steel Engineers Association, April 1995.

Zhang, X. G., 1997. "Corrosion and Electrochemistry of Zinc", Book, Plenum Publication Co., New York, 1996.

Zhang, X. G., 1999. "Corrosion Ratios of Steel to Zinc in Natural Corrosion Environments", Corrosion, Vol. 55, No. 7, pp. 787-794.

Zhang, X. G., 2000a. "Galvanic Corrosion", Chapter 8 in Uhlig's Corrosion Handbook, Second Edition, Edited by R. Winston Revie, John Wiley & Sons, Inc.

Zhang, X. G., 2000b. "Zinc", Chapter 48 in Uhlig Corrosion Handbook, Second Edition, John Wiley & Sons, Inc., New York.

Zhang, X. G., 2000c. "Galvanic protection distance of zinc-coated steels under various environmental conditions", Corrosion, Vol. 56, No. 2, pp. 139-143.

Zhang, X. Gregory, 2000d. "Mechanism of Corrosion Protection by a Zinc Coating", Product Technology Centre, Cominco Ltd., Mississauga, Canada, pp. 5.

Zhang, X. Gregory and Hwang, James, 2000. "Zinc Coating Life Prediction", Intergalva 2000, Berlin, pp.8.

Additional Literature

ASTM A 653/A 653M - 97a, 1997, "Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process", American Society for Testing and Materials.

ASTM C 954 - 98, 1998, "Standard Specification for Steel Drill Screws for the Application of Gypsum Panel Products of Metal Plaster Bases to Steel Studs from 0.033 in. (0.84 mm) to 0.112 in. (2.84 mm) in Thickness", American Society for Testing and Materials.

ASTM A 955 - 98, 1998, "Standard Specification for Load-Bearing (Transverse and Axial) Steel Studs, Runners (Tracks), and Bracing or Bridging for Screw Application of Gypsum Panel Products and Metal Plaster Bases", American Society for Testing and Materials.

ASTM D 1761 - 88 (Reapproved 1995), 1995, "Standard Test Methods for Mechanical Fasteners in Wood", American Society for Testing and Materials.

ITW Buildex, 1997, "Management of Corrosion".

ITW Buildex, 1998, "Corrosion Performance Report.

ITW Buildex, 1999, "Corrosion Performance Report (Updated 1999).

ITW Buildex, 1998a, "Field Report: Fulham Correctional Centre, Sale, Victoria".

ITW Buildex, 1998b, "Field Report: Buildex Construction Fasteners".